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# Stress-induced anisotropy of small strain shear modulus in saturated and unsaturated cohesive soils

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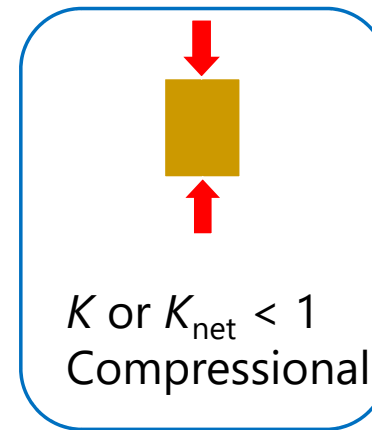
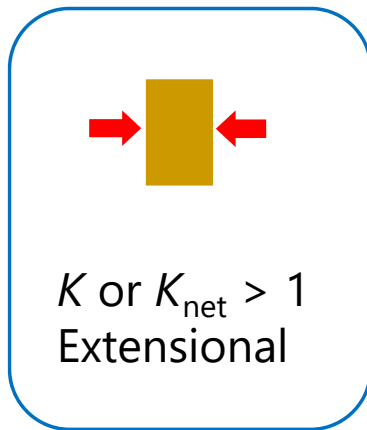
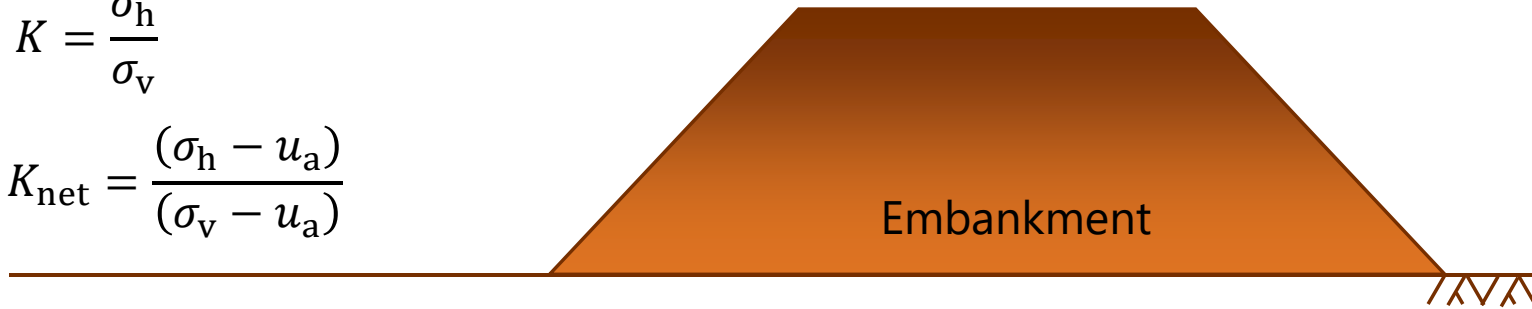
- Background and objectives
- Experiment
- Strain dependency of  $G_{\text{sec}}$
- Anisotropic  $G_0$
- Normalization methods
- Conclusions

# Background

Saturated:  $K = \frac{\sigma_h}{\sigma_v}$

Unsaturated:  $K_{net} = \frac{(\sigma_h - u_a)}{(\sigma_v - u_a)}$

Anisotropic consolidation



Out of scope

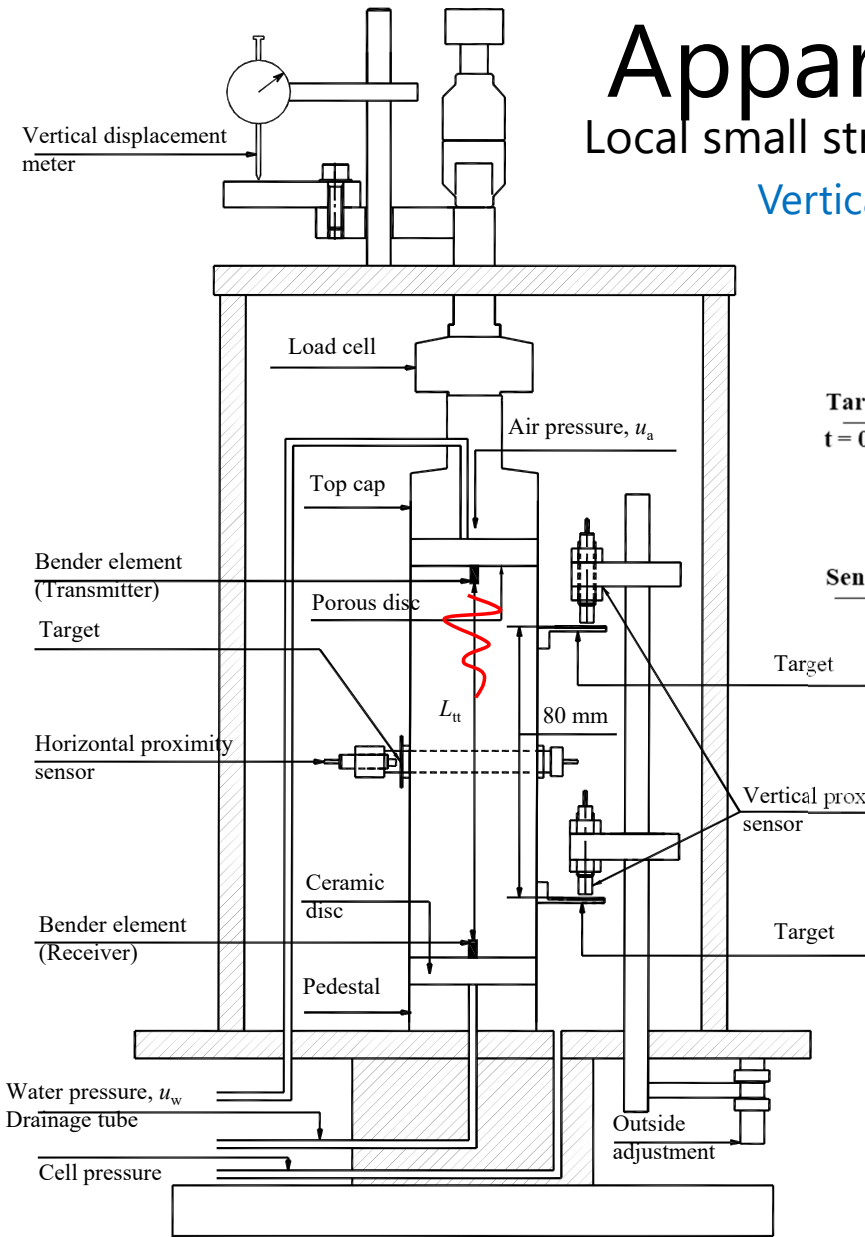
Intermediate: shear stress or inclined loading

# Objectives

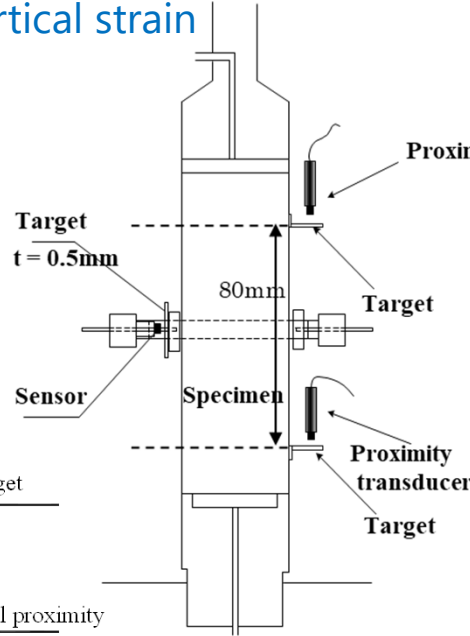
- To create different  $K$  or  $K_{\text{net}}$ -consolidated specimens  
(Anisotropic consolidation)
- To measure  $G_0$  and  $G_{\text{sec}}$   
(Stress-induced anisotropy)
- Normalize  $G_0$  to remove effects of  $K$  or  $K_{\text{net}}$   
(for prediction)

# Apparatus used

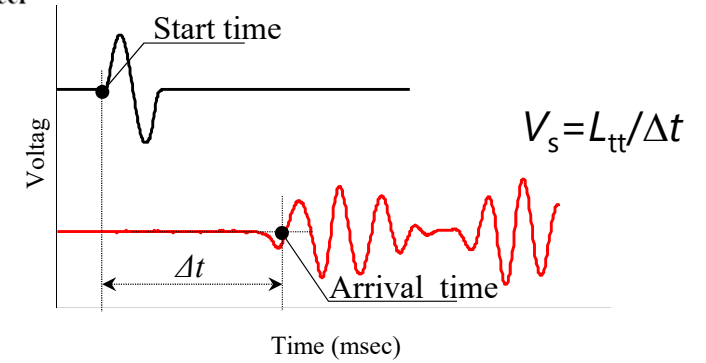
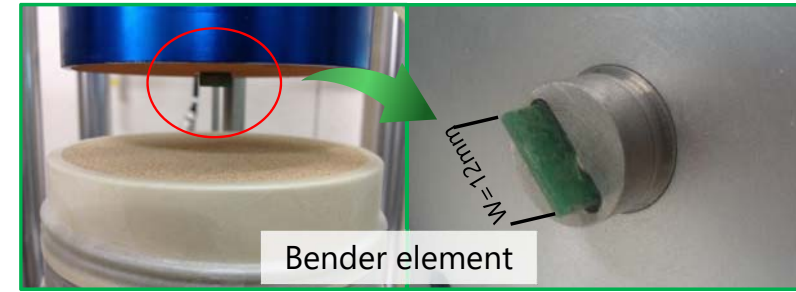
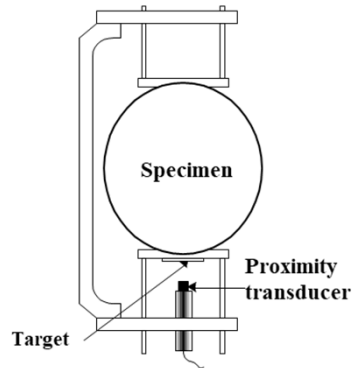
## Local small strain (LSS) measurement



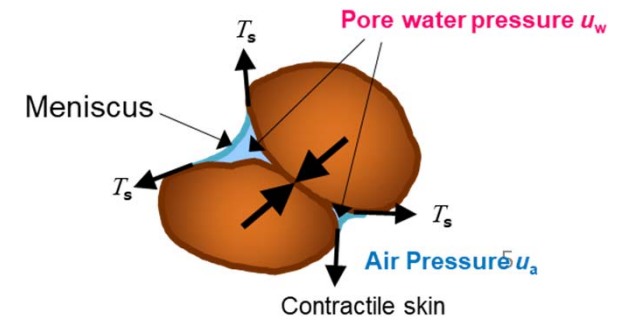
Vertical strain



Horizontal strain

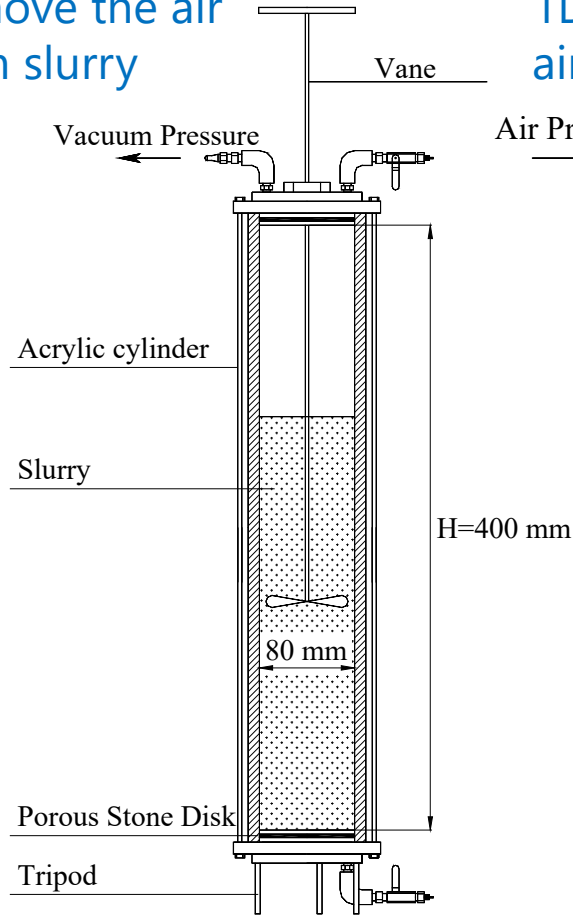


## Unsaturated soils: Pressure plate method



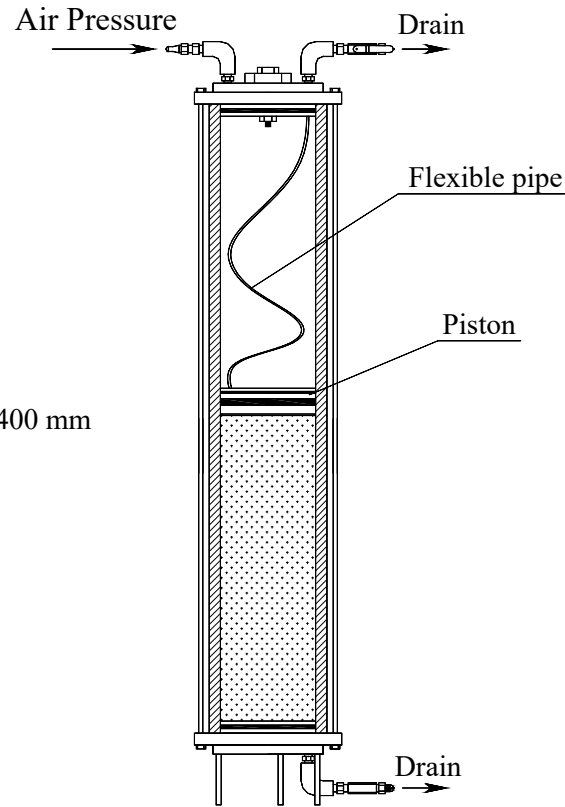
# Specimen preparation

Remove the air from slurry



Step 1  
Under Vacuum Process

1D-consolidation by air pressure



Step 2  
Under One Dimension  
Pre-Consolidation Process

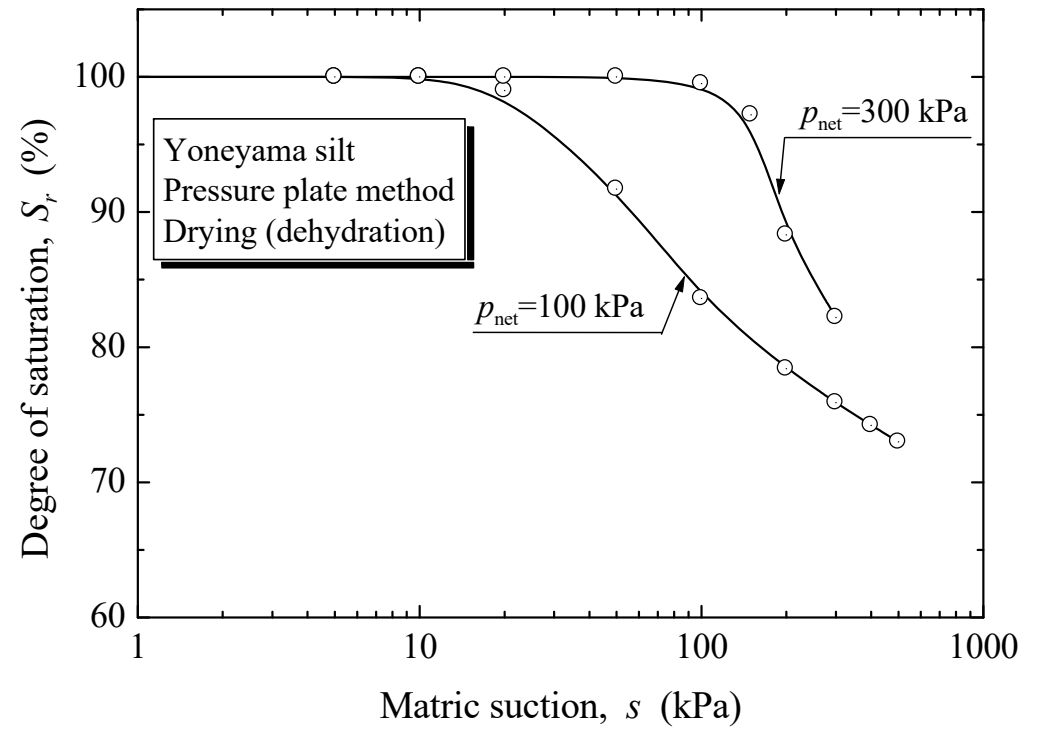
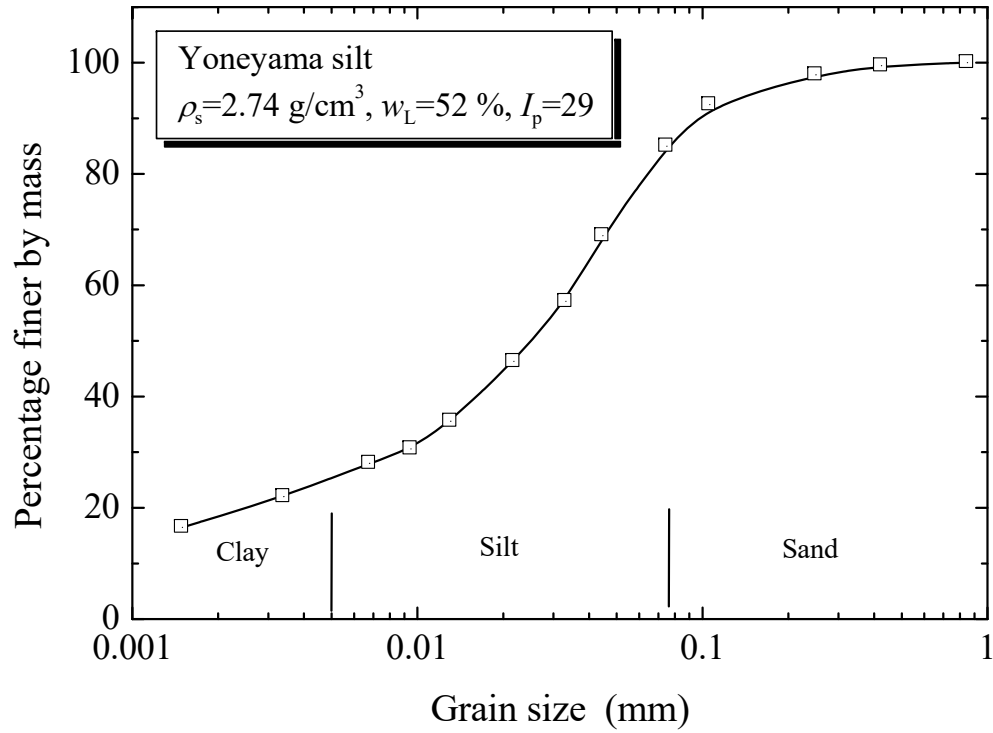


Extracted soil block

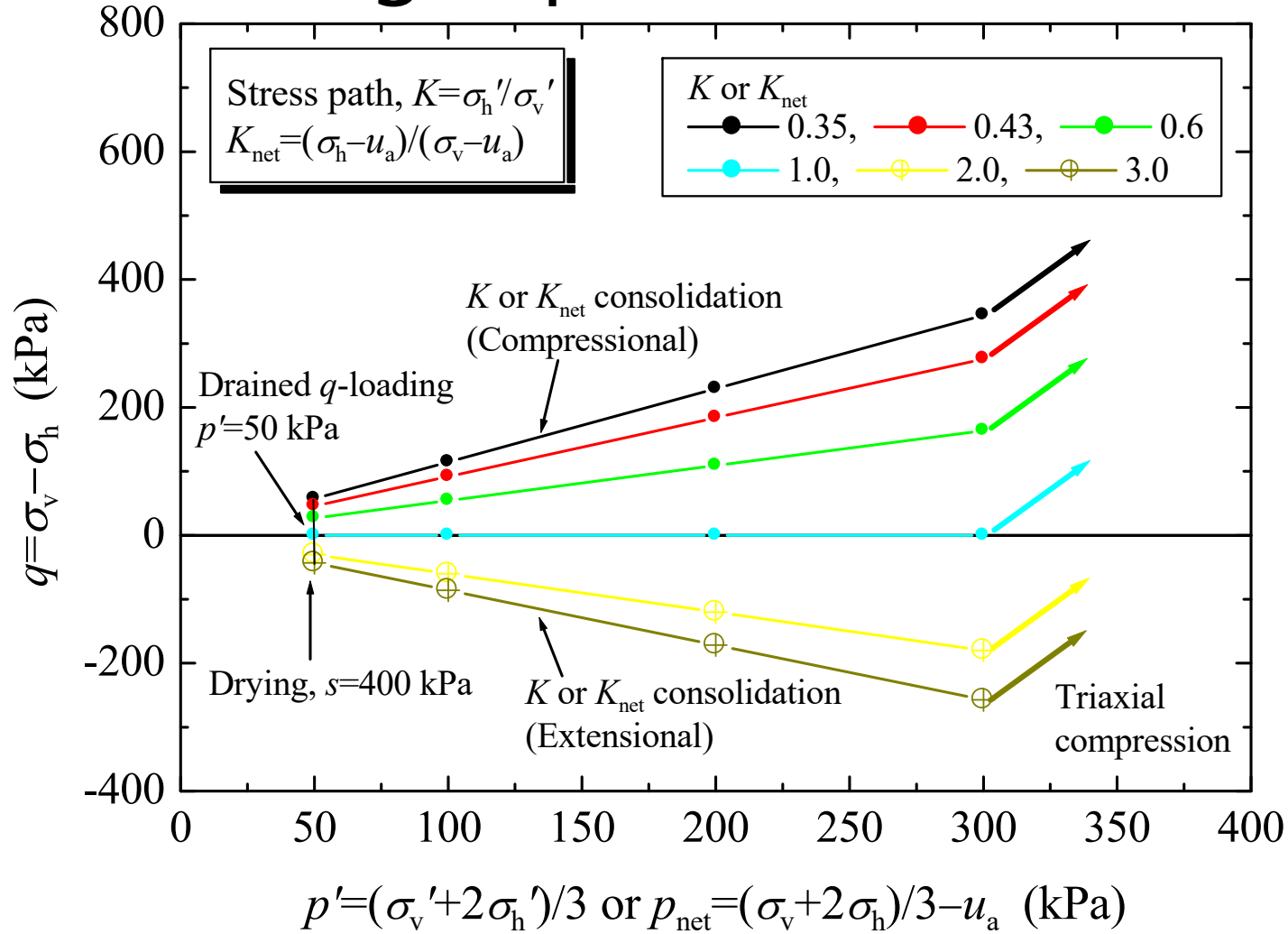


Trimmed  
125 mm height  
50 mm diameter

# Material used



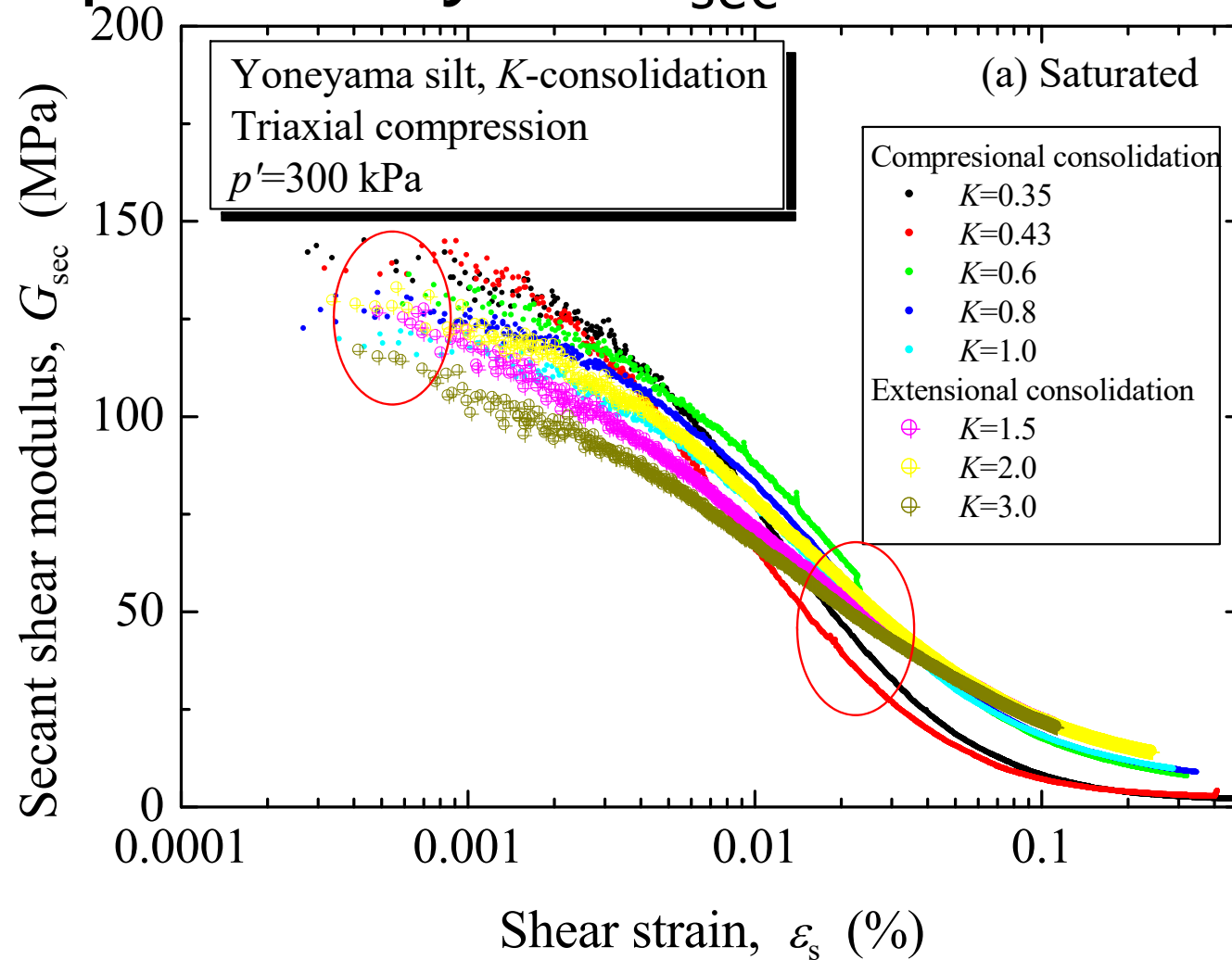
# Stress path during experiment





# Strain dependency of $G_{\text{sec}}$

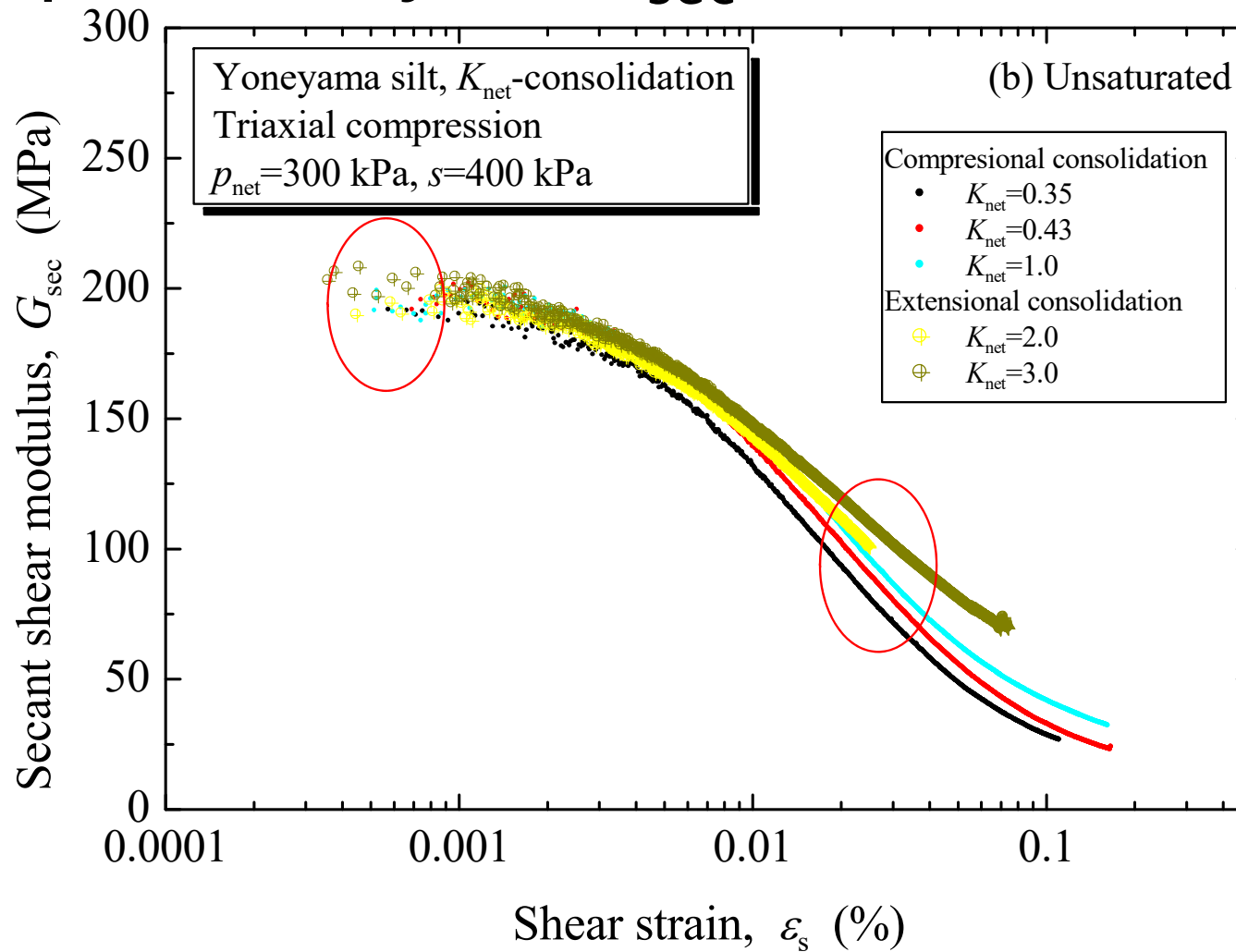
Saturated soil



$$G_{\text{sec}} = \frac{\Delta q}{3\Delta\varepsilon_s}$$

# Strain dependency of $G_{\text{sec}}$

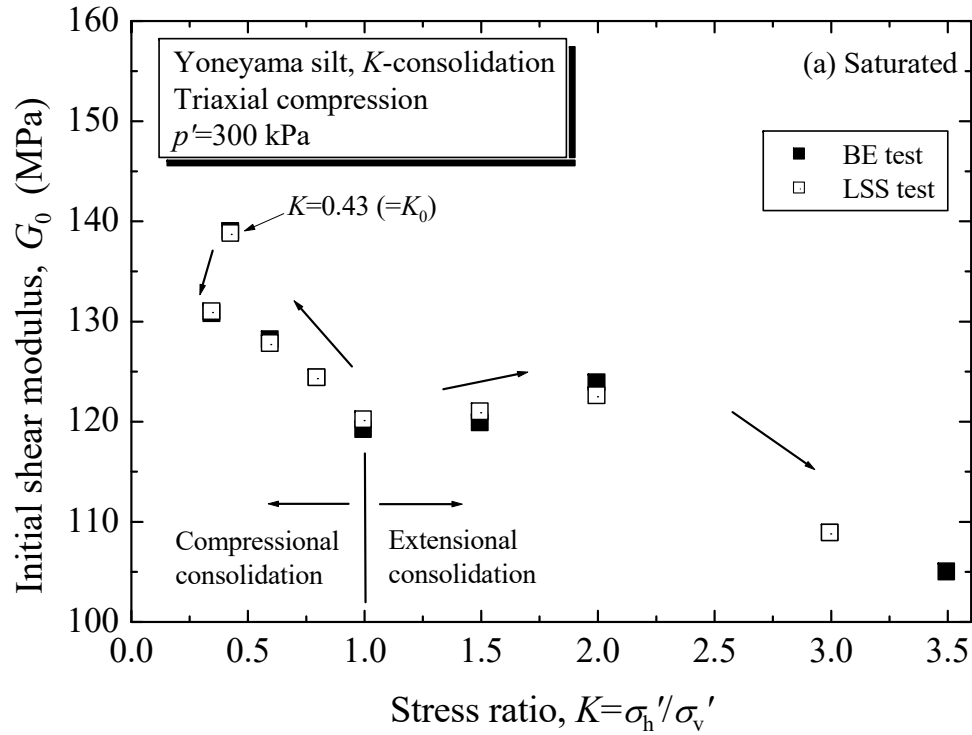
Unsaturated soil



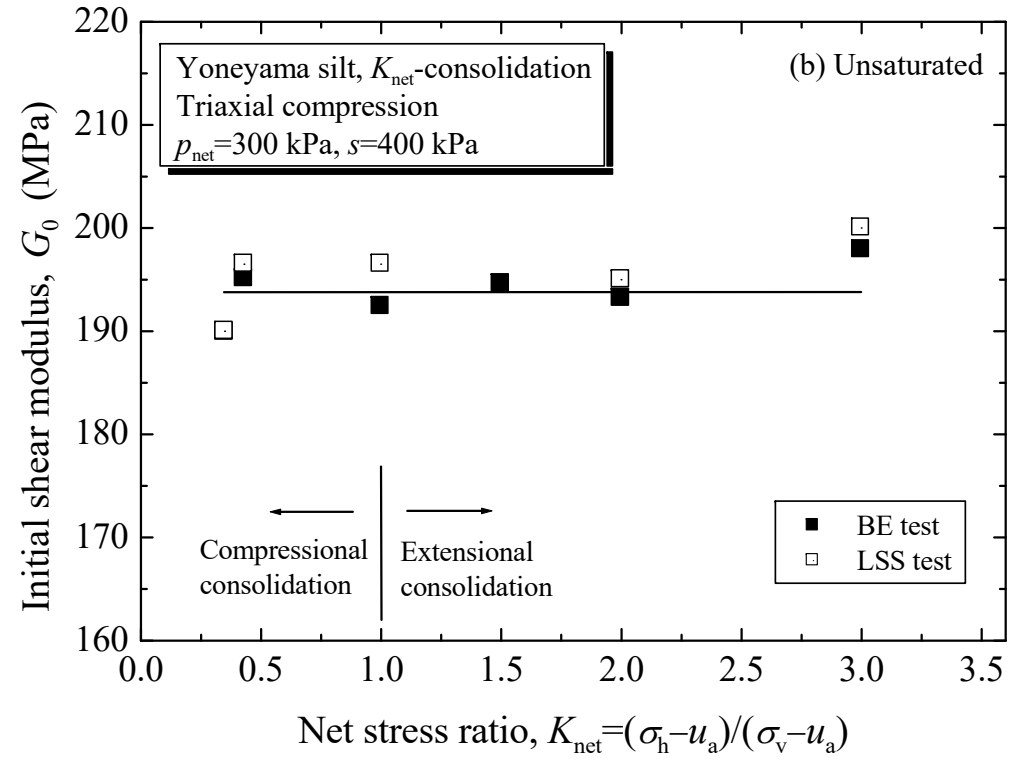
$$G_{\text{sec}} = \frac{\Delta q}{3\Delta\varepsilon_s}$$

# Anisotropic $G_0$

## LSS vs BE tests

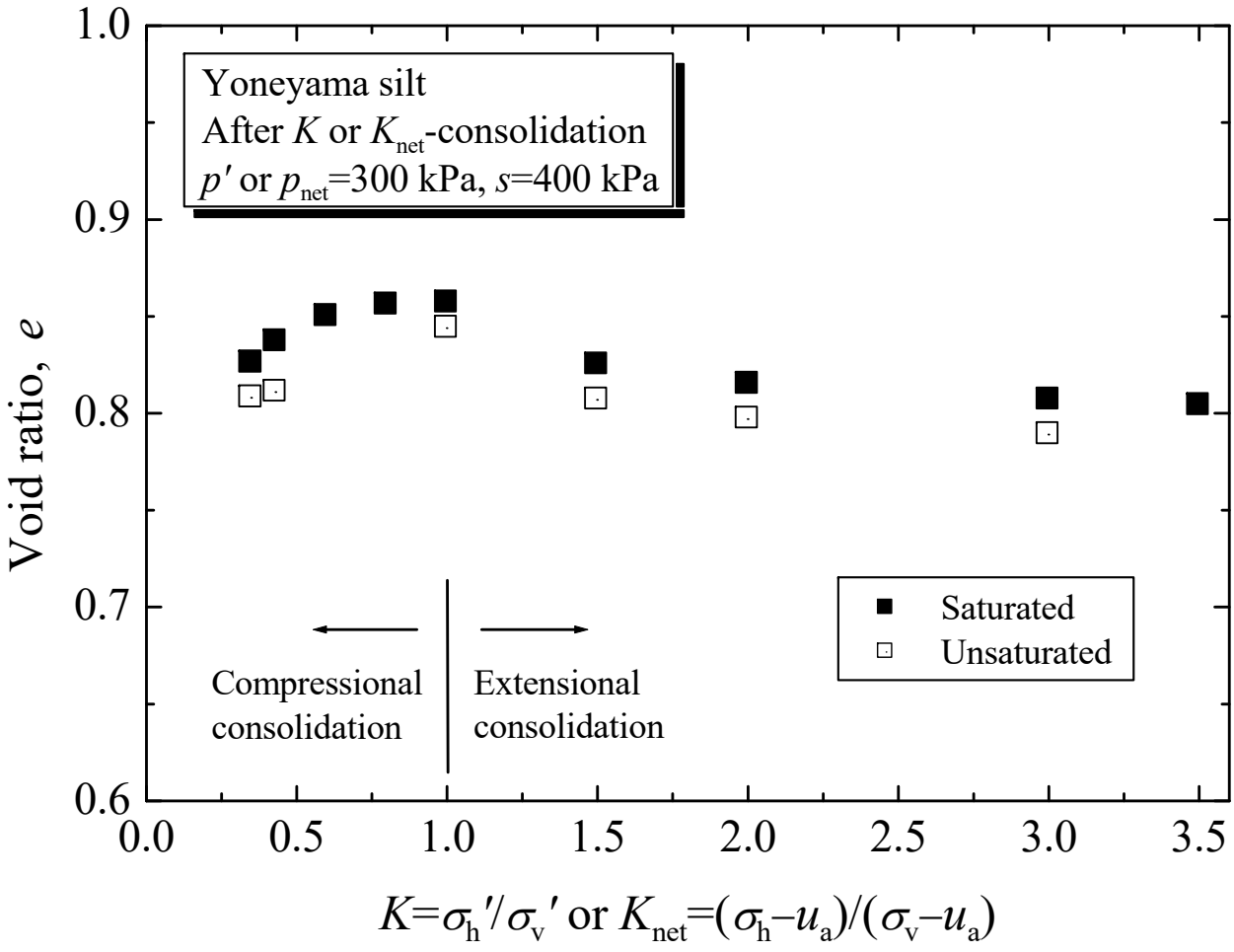


Saturated soil



Unsaturated soil

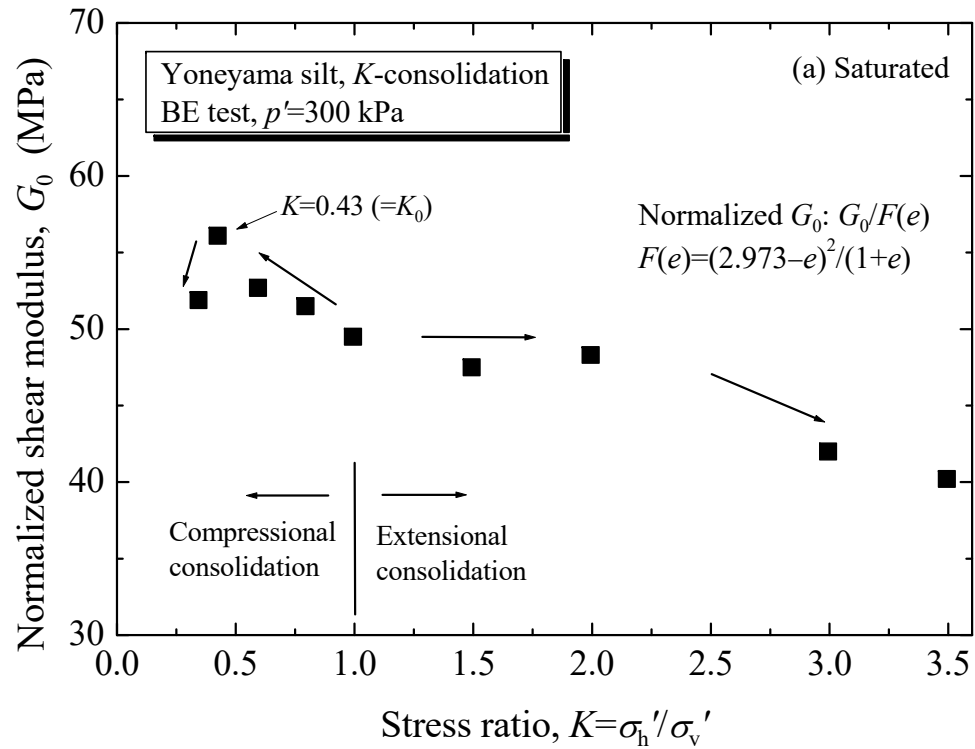
# Void ratio function, $F(e)$



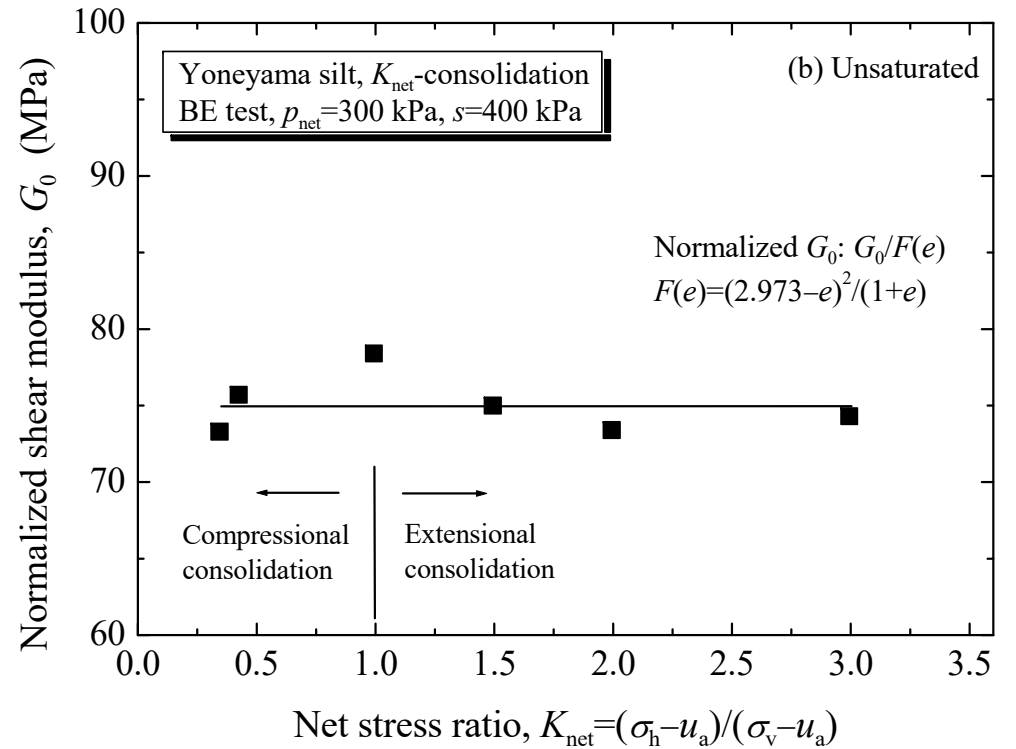
$$F(e) = \frac{(2.973 - e)^2}{(1 + e)}$$

Reported by  
Hardin and Black (1968 and 1969)

# Normalized $G_0$ by $F(e)$



Saturated soil



Unsaturated soil

# Stress functions

Unsaturated soil

$$G_0 = AF(e) \underline{p_a} \left( \frac{p'}{p_a} \right)^n$$

Reported by  
Hardin and Richart (1963) and  
Hardin and Drnevich (1972)

$$n = 0.5$$

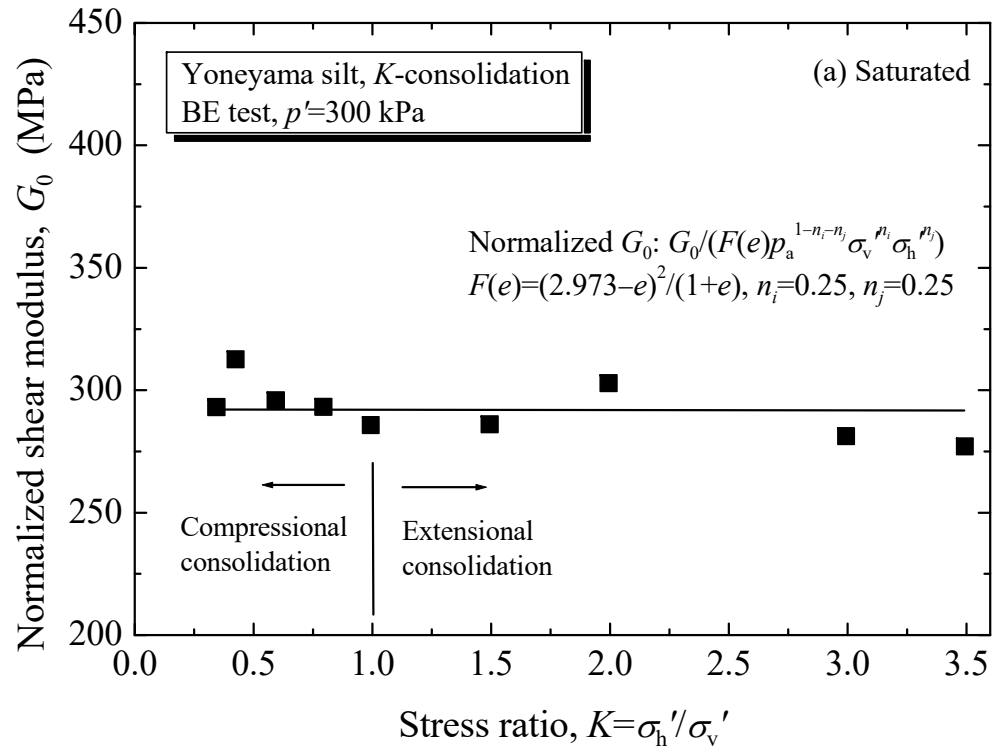
Saturated soil

$$G_0 = S_{ij} F(e) \underline{p_a^{1-n_i-n_j} \sigma'_v{}^{n_i} \sigma'_h{}^{n_j}}$$

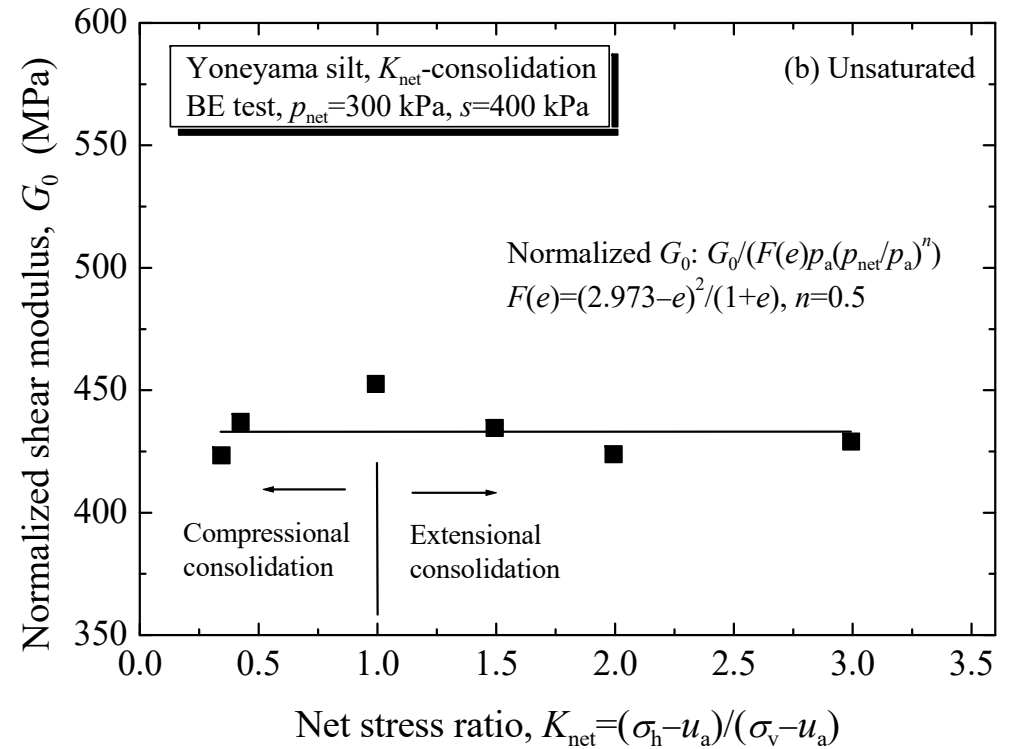
Reported by  
Hardin and Blandford (1989) and  
Jamiolkowski et al. (1995)

$$n_i = 0.25, \quad n_j = 0.25$$

# Normalized $G_0$ by $F(e)$ and stress function



Saturated soil



Unsaturated soil

# Conclusions

- **Very small shear strain:**  $G_0$  is strongly affected by anisotropic stress conditions in the saturated cohesive soil. However, those were not significant for the unsaturated cohesive soil.
- **Shear strain greater than 0.03%:** The trends of  $G_{sec}$  are changed from the trends of  $G_0$ . Degradation with shear strain is large in small  $K$  value.
- The values of  $G_0$  under anisotropic stress conditions are normalized successfully using the void ratio function and the stress function.



# Recommended normalized functions

$$F(e) = \frac{(2.973 - e)^2}{(1 + e)}$$

Unsaturated soil

$$G_0 = AF(e)p_a \left( \frac{p_{\text{net}}}{p_a} \right)^{0.5}$$

Saturated soil

$$G_0 = S_{ij}F(e)p_a^{0.5} \sigma'_v{}^{0.25} \sigma'_h{}^{0.25}$$

Thank you for your kind attention!